

120404	Truss	245212T	Truss Type	FINK	Qty	1	Ply	1	Job Reference (optional)
Saratoga Lumber Traders, Ballston Spa, NY 12020									
Run: 7:250 s Aug 25 2011 Print: 7:410 s Mar 11 2013 MiTek Industries, Inc. Wed Apr 17 13:02:33 2013 Page 1									
ID: Q3wJENK1_6ng7GOXAbL8zHsn-0lWNSC0c3V7kMOLLGatgmKEe01gmYbxwJfONsZpPnK									
-1-0-0	6-9-13	12-0-0	5-2-3	17-2-3	5-2-3	24-0-0	6-9-13	25-0-0	1-0-0

Scale = 1:42.4

Plate Offsets (X,Y): [B:0-0-0,0-0-4], [F:Edge,0-0-4], [H:0-2-0,0-1-11], [J:0-2-0,0-1-11]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 46.2 (Ground Snow=60.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2006/TPI2002	TC 0.79 BC 0.70 WB 0.35 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.18 B-J >999 240 Vert(TL) -0.42 B-J >674 180 Horz(TL) 0.10 F n/a n/a	MT20	169/123

Weight: 77 lb FT = 20%

LUMBER
TOP CHORD 2x4 SPF 1650F 1.5E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF-S No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 2-7-11 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=1698/0-3-8 (min. 0-2-11), F=1698/0-3-8 (min. 0-2-11)
Max Horz B=79(LC 7)
Max Uplift B=-147(LC 9), F=-147(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD B-K=-3113/225, C-K=-2820/228, C-D=-2752/237, D-E=-2752/237, E-L=-2820/228, F-L=-3113/225
BOT CHORD B-J=-130/2739, I-J=-24/1850, H-I=-24/1850, F-H=-130/2739
WEBS C-J=-817/157, D-J=-53/1110, D-H=-53/1110, E-H=-817/157

NOTES
1) Wind: ASCE 7-05; 90mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint B and 147 lb uplift at joint F.
10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Scale = 1:52.2

Plate Offsets (X,Y): [B:0-2-13,0-0-2], [H:0-2-11,0-0-2]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 53.9 (Ground Snow=70.0) TCDL 8.0 BCLL 0.0 * BCDL 7.0	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2006/TPI2002	TC 0.75 BC 0.78 WB 0.41 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.40 J-L >829 240 Vert(TL) -0.50 J-L >661 180 Horz(TL) 0.13 H n/a n/a	MT20 MT20H	169/123 187/143

Weight: 115 lb FT = 20%

LUMBER
TOP CHORD 2x4 SYP SS
BOT CHORD 2x4 SYP No.1
WEBS 2x4 SPF-S No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 2-7-13 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=2230/0-3-8 (min. 0-2-10), H=2230/0-3-8 (min. 0-2-10)
Max Horz B=105(LC 8)
Max Uplift B=-290(LC 9), H=-290(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD B-O=-3993/409, C-O=-3647/411, C-D=-3399/357, D-E=-3220/379, E-F=-3220/379, F-G=-3399/357, G-P=-3647/411, H-P=-3993/409
BOT CHORD B-L=-275/3524, L-M=-99/2346, K-M=-99/2346, K-N=-99/2346, J-N=-99/2346, H-J=-275/3524
WEBS C-L=-1097/195, E-L=-87/1287, E-J=-87/1287, G-J=-1097/195

NOTES
1) Wind: ASCE 7-05; 90mph; TCCL=4.8psf; BCCL=4.2psf; h=25ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=70.0 psf (ground snow); Ps=53.9 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 53.9 psf on overhangs non-concurrent with other live loads.
6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
7) All plates are MT20 plates unless otherwise indicated.
8) The Fabrication Tolerance at joint B = 10%; joint L = 10%; joint J = 10%; joint H = 10%.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 7.0psf.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint B and 290 lb uplift at joint H.
12) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
13) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Scale = 1:48.9

Plate Offsets (X,Y): [B:0-0-0,0-0-4], [H:0-0-0,0-0-4]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 46.2 (Ground Snow=60.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2006/TPI2002	TC 0.87 BC 0.87 WB 0.41 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.43 J-L >716 240 Vert(TL) -0.73 J-L >425 180 Horz(TL) 0.12 H n/a n/a	MT20 MT20H	169/123 148/108

Weight: 91 lb FT = 20%

LUMBER
TOP CHORD 2x4 SPF 1650F 1.5E "Except"
T2: 2x4 SYP No.1
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF-S No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=1996/0-3-8 (min. 0-3-2), H=1996/0-3-8 (min. 0-3-2)
Max Horz B=94(LC 7)
Max Uplift B=-193(LC 9), H=-193(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD B-C=-3571/221, C-D=-3191/199, D-E=-3167/229, E-F=-3167/229, F-G=-3191/199, G-H=-3571/221
BOT CHORD B-L=-102/3155, L-M=-3/2075, K-M=-3/2075, K-N=-3/2075, H-J=-102/3155
WEBS C-L=-846/158, E-L=-28/1299, E-J=-28/1299, G-J=-846/158

NOTES
1) Wind: ASCE 7-05; 90mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; B=45ft; L=26ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint B and 193 lb uplift at joint H.
11) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Scale = 1:45.7

Plate Offsets (X,Y): [B:0-0-0,0-0-4], [H:0-1-14,0-1-14], [J:0-1-14,0-1-14]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 46.2 (Ground Snow=60.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2006/TPI2002	TC 0.87 BC 0.63 WB 0.34 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.43 H-J >999 240 Vert(TL) -0.38 B-J >756 180 Horz(TL) 0.13 F n/a n/a	MT20	169/123

Weight: 80 lb FT = 20%

LUMBER
TOP CHORD 2x4 SPF 1650F 1.5E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF-S No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 3-0-14 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=1811/0-3-8 (min. 0-2-13), F=1811/0-3-8 (min. 0-2-13)
Max Horz B=85(LC 7)
Max Uplift B=-184(LC 9), F=-184(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD B-K=-3039/170, C-K=-2775/199, C-D=-2681/208, D-E=-2681/208, E-L=-2775/199, F-L=-3039/170
BOT CHORD B-J=-79/2662, I-J=0/1813, H-I=0/1813, F-H=-79/2662
WEBS C-J=-806/145, D-J=-35/1084, D-H=-35/1084, E-H=-806/145

NOTES
1) Wind: ASCE 7-05; 90mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint B and 184 lb uplift at joint F.
10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Scale = 1:42.4

Plate Offsets (X,Y): [D:0-3-0,0-1-12], [H:0-1-12,0-2-0], [J:0-1-12,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 46.2 (Ground Snow=60.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2006/TPI2002	TC 0.81 BC 0.79 WB 0.33 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.56 H-J >999 240 Vert(TL) -0.25 F-H >515 180 Horz(TL) 0.13 F n/a n/a	MT20	169/123

Weight: 74 lb FT = 20%

LUMBER
TOP CHORD 2x4 SPF 1650F 1.5E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF-S No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 2-5-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) B=1698/0-3-8 (min. 0-2-11), F=1698/0-3-8 (min. 0-2-11)
Max Horz B=52(LC 8)
Max Uplift B=-147(LC 9), F=-147(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD B=-3736/269, C-K=-3527/282, C-D=-3157/229, D-E=-3157/229, E-L=-3527/282, F-L=-3736/269
BOT CHORD B-J=-204/3435, I-J=-72/2322, H-I=-72/2322, F-H=-204/3435
WEBS C-J=-836/161, D-J=-25/1054, D-H=-25/1054, E-H=-836/161

NOTES
1) Wind: ASCE 7-05; 90mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
3) Roof design snow load has been reduced to account for slope.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint B and 147 lb uplift at joint F.
10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard