



Plate Offsets (X,Y): [B:0-11-10,0-0-2], [B:0-3-4,1-1-6], [F:0-3-4,1-1-6], [F:Edge,0-0-2], [I:0-3-0,0-0-4]

| LOADING (psf) | SPACING | CSI | DEFL | in (loc) | l/defl | L/d | PLATES | GRIP |
|---------------------------------|----------------------|----------|----------------|----------|--------|-----|----------------|----------|
| TCLL 42.6 (Ground Snow=70.0) | Plates Increase 1.15 | TC 0.37 | Vert(LL) -0.22 | H-J | >999 | 240 | MT20 | 169/123 |
| TCDL 5.0 | Lumber Increase 1.15 | BC 0.82 | Vert(TL) -0.30 | B-J | >940 | 180 | | |
| BCLL 0.0 * | Rep Stress Incr NO | WB 0.48 | Horz(TL) 0.13 | F | n/a | n/a | | |
| BCDL 3.0 | Code IBC2006/TPI2002 | (Matrix) | | | | | Weight: 122 lb | FT = 20% |

LUMBER
 TOP CHORD 2x6 SYP SS
 BOT CHORD 2x4 SYP No.1
 WEBS 2x4 SPF-S No.2
 WEDGE
 Left: 2x4 SPF-S No.2, Right: 2x4 SPF-S No.2

BRACING
 TOP CHORD 2-0-0 oc purlins (3-7-5 max.)
 (Switched from sheeted: Spacing > 2-0-0).
 BOT CHORD Rigid ceiling directly applied or 7-1-4 oc bracing.

REACTIONS (lb/size) B=2616/0-3-8 (min. 0-3-2), F=2616/0-3-8 (min. 0-3-2)
 Max Horz B=-158(LC 7)
 Max Uplift B=-648(LC 9), F=-648(LC 9)
 Max Grav B=2628(LC 2), F=2628(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD B-K=-4826/1064, C-K=-4340/1066, C-D=-4021/1005, D-E=-4021/1005, E-L=-4340/1066,
 F-L=-4826/1064
 BOT CHORD B-J=-849/4227, I-J=-448/2803, H-I=-448/2803, F-H=-849/4227
 WEBS C-J=-1405/372, D-J=-297/1515, D-H=-297/1515, E-H=-1405/372

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCDL=3.0psf; BCDL=1.8psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-05; Pg=70.0 psf (ground snow); Ps=42.6 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1; Unobstructed slippery surface
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 53.9 psf on overhangs non-concurrent with other live loads.
 - 6) The bottom chord dead load shown is sufficient only to cover the truss weight itself and does not allow for any additional load to be added to the bottom chord.
 - 7) Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
 - 8) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 648 lb uplift at joint B and 648 lb uplift at joint F.
 - 12) Following joints to be plated by qualified designer: Joint(s) B, F, not plated.
 - 13) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
 - 14) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.