

LOADING (psf)		SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	46.2	2-0-0	TC	Vert(LL)	-0.20	B-H	>999	MT20	169/123
(Ground Snow=60.0)		Plates Increase	BC	Vert(TL)	-0.53	B-H	>442		
TCDL	10.0	Lumber Increase	WB	Horz(TL)	0.08	F	n/a		
BCLL	0.0 *	Rep Stress Incr	(Matrix)					Weight: 60 lb	FT = 20%
BCDL	10.0	Code IBC2006/TPI2002							

LUMBER	BRACING
TOP CHORD	TOP CHORD
BOT CHORD	BOT CHORD
WEBS	

Structural wood sheathing directly applied or 2-7-6 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

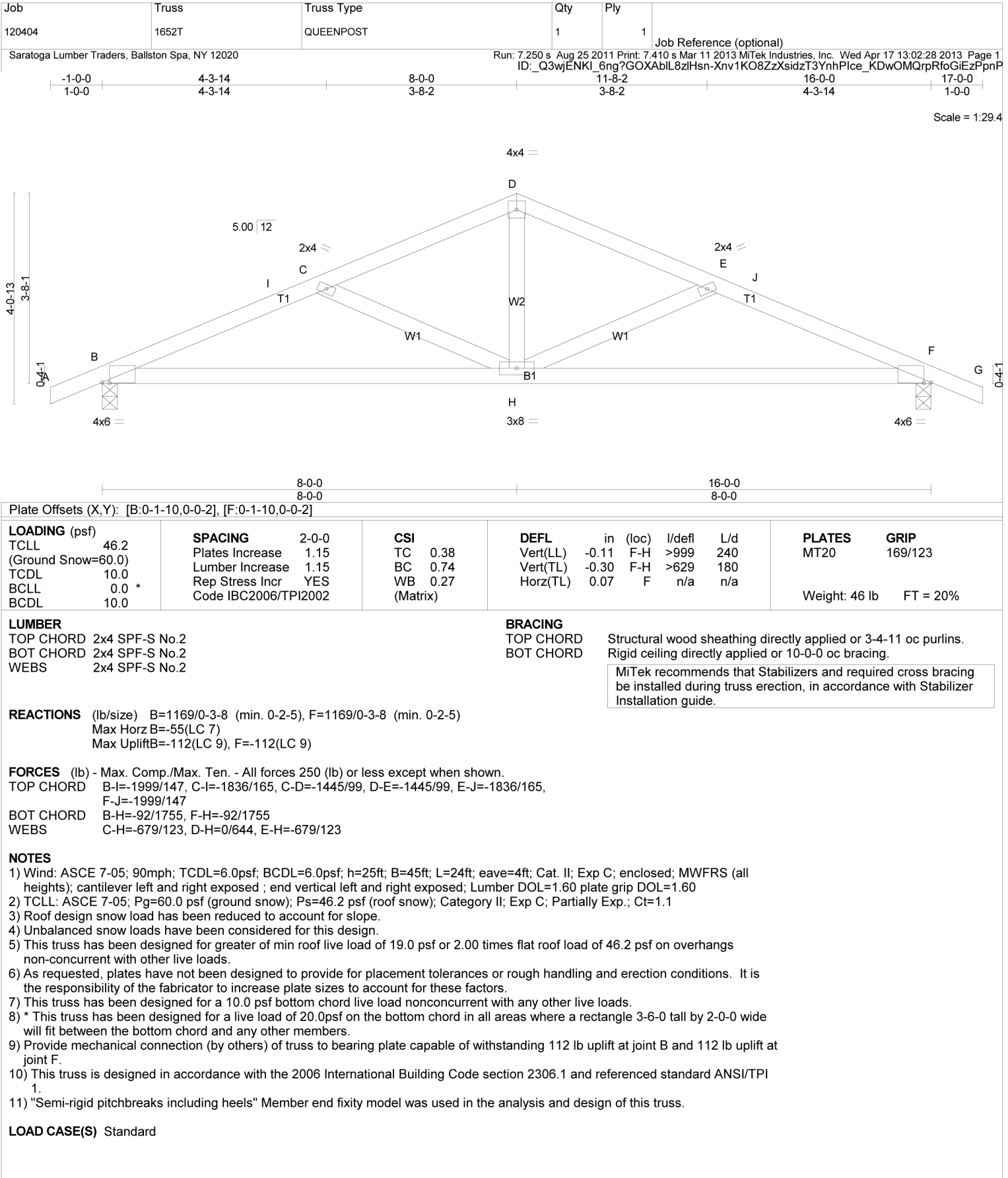
REACTIONS (lb/size)	B=1433/0-3-8 (min. 0-2-4), F=1433/0-3-8 (min. 0-2-4)
	Max Horz B=-67(LC 7)
	Max Uplift B=-129(LC 9), F=-129(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD
B-I=-2552/213, C-I=-2324/215, C-D=-1837/129, D-E=-1837/129, E-J=-2324/215, F-J=-2552/213
BOT CHORD
B-H=-130/2249, F-H=-130/2249
WEBS
C-H=-888/162, D-H=-2/857, E-H=-888/162

NOTES

- 1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint B and 129 lb uplift at joint F.
- 10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)		SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	46.2	2-0-0	TC	Vert(LL)	-0.11	F-H	>999	MT20	169/123
(Ground Snow=60.0)		Plates Increase	BC	Vert(TL)	-0.30	F-H	>629		
TCDL	10.0	Lumber Increase	WB	Horz(TL)	0.07	F	n/a		
BCLL	0.0 *	Rep Stress Incr	(Matrix)					Weight: 46 lb	FT = 20%
BCDL	10.0	Code IBC2006/TPI2002							

LUMBER	BRACING
TOP CHORD	TOP CHORD
BOT CHORD	BOT CHORD
WEBS	

Structural wood sheathing directly applied or 3-4-11 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size)	B=1169/0-3-8 (min. 0-2-5), F=1169/0-3-8 (min. 0-2-5)
	Max Horz B=-55(LC 7)
	Max Uplift B=-112(LC 9), F=-112(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD
B-I=-1999/147, C-I=-1836/165, C-D=-1445/99, D-E=-1445/99, E-J=-1836/165, F-J=-1999/147
BOT CHORD
B-H=-92/1755, F-H=-92/1755
WEBS
C-H=-679/123, D-H=0/644, E-H=-679/123

NOTES

- 1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=46.2 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 19.0 psf or 2.00 times flat roof load of 46.2 psf on overhangs non-concurrent with other live loads.
- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint B and 112 lb uplift at joint F.
- 10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard