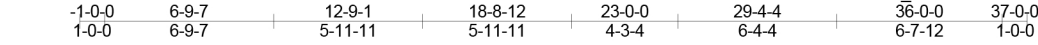


Saratoga Lumber Traders, Ballston Spa, NY 12020

Job Reference (optional)



Scale = 1:92.4

Plate Offsets (X,Y): [B:0-3-3,Edge], [F:0-3-3,0-2-8], [J:0-1-8,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 35.8 ** (Ground Snow=50.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IBC2006/TPI2002	TC 0.96 BC 0.53 WB 0.55 (Matrix-M)	in (loc) l/defl L/d Vert(LL) -0.17 R >999 240 Vert(TL) -0.35 R-U >792 180 Horz(TL) 0.07 L n/a n/a	MT20	169/123
Weight: 148 lb FT = 20%					

LUMBER
 TOP CHORD 2x4 SPF-S No.2
 BOT CHORD 2x4 SYP No.1
 WEBS 2x4 SPF-S No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except 6-0-0 oc bracing: O-P.
 WEBS 1 Row at midpt E-P, H-O

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) B=1342/0-5-8 (min. 0-1-11), O=2353/0-3-8 (min. 0-2-12), L=608/0-5-8 (min. 0-1-8)
 Max HorzB=-333(LC 9)
 Max UpliftB=-133(LC 9), O=-30(LC 9), L=-231(LC 9)
 Max GravB=1452(LC 2), O=2353(LC 1), L=651(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD B-C=-2917/262, C-D=-2354/174, D-V=-2268/175, E-V=-2172/188, E-F=-734/125, F-G=-776/136,
 G-W=-44/1413, H-W=-89/1229, H-I=-795/324, I-J=-1138/298, J-L=-716/281
 BOT CHORD B-R=-17/2683, Q-R=-104/1757, P-Q=-104/1757, O-P=-817/392, N-O=-66/947, M-N=-94/908, L-M=-50/260
 WEBS C-R=-673/166, E-R=0/707, E-P=-1487/173, G-P=0/1477, G-O=-2449/222, H-O=-1674/494, J-M=-38/625

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=36ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) ** TCLL: ASCE 7-05; Pg=50.0 psf (ground snow); Ps= varies (min. roof snow=35.8 psf) see load cases; Category II; Exp C; Partially Exp.; Ct=1.1
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 38.5 psf on overhangs non-concurrent with other live loads.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Bearing at joint(s) L considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint B, 30 lb uplift at joint O and 231 lb uplift at joint L.
 - 10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
 - 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard
 1) Snow: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-F=-97, F-J=-92, J-K=-92, O-S=-20, L-O=-20